

1. THE NOTES ON THIS SHEET APPLY TO ALL WORKS UNDER THIS CONTRACT UNLESS OTHERWISE NOTED ON THE PLAN AND PROFILE DRAWINGS AND/OR SPECIFIC DETAIL DRAWINGS.
2. ALL MATERIALS ARE TO COMPLY WITH THE MANUFACTURER'S SPECIFICATIONS PER APPROVED SUPPLIERS LISTED IN SCHEDULE "B" OF THE STANDARD DRAWINGS.
3. THE STANDARD DRAWINGS OF THE TOWN OF WHITCHURCH-STOUFFVILLE, ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS (OPSS) AND THE ONTARIO PROVINCIAL STANDARD DRAWINGS (OPSD) CONSTITUTE PART OF THE PLANS OF THIS CONTRACT. ORDER OF PRECEDENCE OF THE STANDARD DRAWINGS IS: 1. THIS CONTRACT, 2. THE TOWN OF WHITCHURCH-STOUFFVILLE AND SECONDLY ONTARIO PROVINCIAL STANDARD DRAWINGS.
4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN ALL RELEVANT STANDARD DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION.
5. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS SHOWN ON THE ENGINEERING DRAWINGS ARE IN METRES OR MILLIMETRES.
6. ALL DIMENSIONS AND ELEVATIONS SHALL BE CHECKED AND VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO CONSTRUCTION AND ANY DISCREPANCIES SHALL BE REPORTED IMMEDIATELY TO THE ENGINEER.
7. EXISTING SERVICES AND UTILITIES SHOWN ON THESE CONTRACT DRAWINGS ARE BASED ON THE INFORMATION PROVIDED BY THE TOWN OF WHITCHURCH-STOUFFVILLE. NOT GUARANTEED. THE CONTRACTOR SHALL INTERPRET THIS INFORMATION AS HE WISHES WITH THE UNDERSTANDING THAT THE OWNER DISCLAIMS ALL RESPONSIBILITY FOR ITS ACCURACY AND/OR SUFFICIENCY.
8. ALL EXISTING UTILITIES SHALL BE PROTECTED AND MAINTAINED. ANY UTILITIES TO BE REMOVED OR PLACED UNDERGROUND IN LOCATIONS SHOWN ON THE ROAD SECTIONS LISTED IN THE STANDARD DRAWINGS.
9. EROSION AND EROSION PROTECTION DEVICES ARE TO BE IN PLACE PRIOR TO THE COMMENCEMENT OF CONSTRUCTION AND SHALL REMAIN IN PLACE AND BE MAINTAINED BY THE CONTRACTOR UNTIL CONSTRUCTION IS COMPLETE AND THE GROUND COVER HAS ESTABLISHED TO PROTECT EXPOSED SOILS.
10. NATIVE MATERIAL, SUITABLE FOR BACKFILL, SHALL BE COMPACTED TO 95% STANDARD PROCTOR MAXIMUM DRY DENSITY.
11. SUBGRADE MATERIAL, SUITABLE FOR BACKFILL, SHALL BE PLACED IN LAYERS 150mm IN DEPTH MAXIMUM AND COMPACTED TO 100% STANDARD PROCTOR MAXIMUM DRY DENSITY.
12. ALL DISTURBED AREAS ARE TO BE REINSTATED TO THEIR ORIGINAL CONDITION OR BETTER, AS APPROVED BY THE TOWN ENGINEER. THE TOPSOIL OF NORTH AND SOUTH END OF ROAD SHALL BE RESTORED BY PLACING 300mm OF APPROVED TOPSOIL AND NURSERY SOD.
13. ALL FENCING SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE TOWN STANDARD DRAWINGS.
14. THE STREET LIGHTING SYSTEM SHALL BE DESIGNED BY A QUALIFIED CONSULTING ENGINEER IN ACCORDANCE WITH THE TOWN OF WHITCHURCH-STOUFFVILLE STANDARD DRAWINGS AND THE LATEST EDITION STANDARDS AND APPROVED BY THE TOWN. STREET LIGHTING SYSTEMS FOR ROADWAYS IN THE TOWN OF WHITCHURCH-STOUFFVILLE SHALL MEET THE REQUIREMENTS OF HYDRO ONE.

1. THE ROAD PAVEMENT MINIMUM STRUCTURE SHALL CONSIST OF THE FOLLOWING AND WILL BE COMPLETED BY A SOLID CONCRETE SLAB APPROVED BY THE TOWN:
 - 40mm of H1.3 SURFACE COURSE ASPHALT
 - 50mm of H18 BINDER COURSE ASPHALT
 - 150mm of 10mm CRUSHER RUN LIMESTONE OR EQUIVALENT
 - 40mm of 50mm CRUSHER RUN LIMESTONE OR EQUIVALENT *
2. * RECYCLED CONCRETE MAY BE SUBSTITUTED UPON WRITTEN APPROVAL OF TOWN BOULEVARDS. - 450mm TOPSOIL & SOD
3. DITCHES: 300mm TOPSOIL & SOD
4. NATIVE SUBGRADE TO BE COMPACTED TO MINIMUM 95 STANDARD PROCTOR MAXIMUM DRY DENSITY AND SHALL BE PROOF ROLLED. NATIVE SUBGRADE SHALL HAVE A CROSS-FALL TO THE DRAINAGE DITCHES TO BE MAINTAINED AND SHALL BE PROOF ROLLED AND IS SUBJECT TO APPROVAL BY THE TOWN ENGINEER.
5. CONCRETE CURBRIER CURBS CONFORMING TO OPSD 600.11 SHALL BE USED.
6. CONCRETE STRENGTH IS TO BE MINIMUM OF 30MPa AT 28 DAYS WITH 7% ± 1.5% AIR ENTRAINMENT.
7. DRIVEWAY DEPRESSIONS SHALL BE FORMED IN THE CURB AS PER OPSD 351.01.
8. DRIVEWAY CONSTRUCTION SHALL COMPLY WITH OPSD M310.01 (MODIFIED) AND SHALL BE CONSTRUCTED ON 100mm OF GRANULAR "A" ON POLYETHYLENE BARRIER AND A COMPACTED FOUNDATION. CONCRETE STRENGTH IS TO BE A MINIMUM OF 30MPa AT 28 DAYS WITH 7% ± 1.5% AIR ENTRAINMENT.
9. RESIDENTIAL DRIVEWAYS TO BE CONSTRUCTED WITH A MINIMUM OF 150mm GRANULAR "A" ON POLYETHYLENE BARRIER AND 100mm OF GRANULAR "A" ON 150.3A TOP ASPHALT FROM STREET TO GARAGE/DWELLING, OR AS APPROVED BY THE TOWN ENGINEER.
10. ALL ACCESS DRIVEWAYS SHALL BE LOCATED A MINIMUM OF 1.0M FROM LIGHT POLES, HYDRO TRANSFORMERS, CATCHBASINS, HYDRANTS, WATERMAIN VALVES, BELL MANHOLES, BELL AND MANHOLE COVERS, AND ALL OTHER UTILITY VALVES, BOXES, OR COVERS.
11. DRIVEWAYS WHERE FRONTAGE LIMITATIONS INTERFERE WITH STANDARD LOCATIONS, SITE SPECIFIC DRIVEWAY LOCATIONS SHALL BE DETERMINED BY THE TOWN ENGINEER AND TOWN PLANS.
12. MINIMUM SLOPES ON DRIVEWAYS SHALL BE 1.0% AND MAXIMUM SLOPES SHALL BE 6.0%.
13. ROAD OCCUPANCY PERMIT IS REQUIRED FROM THE TOWN WORKS DEPARTMENT PRIOR TO THE INSTALLATION OF ANY ROAD CLOSURE OR ON ANY ROAD THAT IS EXTERNAL TO THE PLAN. A MINIMUM 48 HOUR NOTICE IS REQUIRED.

1. STORM SEWER TO BE LOCATED TYPICALLY 1.5m TO THE WEST OR SOUTH OF CENTRELINE OF ROAD.
2. PIPE SHALL CONFORM TO THE REQUIREMENTS SHOWN BELOW:
 - a) NON-REINFORCED CONCRETE PIPE, CSA STANDARD A257.1 CLASS 12.3.
 - b) REINFORCED CONCRETE PIPE, CSA STANDARD A257.2 STRENGTH CLASS 60-D, 65-D, 100-D AND 130-D.
 - c) PVC PIPE, CSA STANDARD B182.1 AND B182.4 (SDR35).
3. ALL STORM SEWERS OVER 450mm DIAMETER SHALL BE CONSTRUCTED WITH REINFORCED CONCRETE PIPE.
4. SEWERS SHALL BE CONSTRUCTED WITH BEDDING AS PER OPSD 802.010, AND 802.030, UNLESS OTHERWISE SPECIFIED BY A GEOTECHNICAL CONSULTANT AND APPROVED BY THE TOWN.
5. TRENCH BACKFILL SHOULD BE PLACED IN 200mm LIFTS AND COMPACTED TO 95% SPMD0.
6. ALL STORM SEWER MANHOLES TO BE ACCORDING TO TOWN STANDARDS AND BE BENCHED TO TOWN.
7. DROP STRUCTURES SHALL CONFORM WITH OPSD 1003.010 AND 1003.020.
8. MANHOLE TOPS ARE TO BE SET TO BASE COURSE ASPHALT GRADE AND THEN ADJUSTED TO FINAL GRADE. FRAME AND COVER TO BE PER OPSD M401.010, TYPE 'B', GRADE AND COVER TO BE ADJUSTED AS NECESSARY TO ACCOMMODATE PRODUCTS SPECIFICALLY MANUFACTURED FOR THAT PURPOSE. MODULAR ADJUSTMENT RINGS ARE TO BE USED TO ADJUST THE MANHOLE TO FINAL GRADE.
9. CATCHBASINS MUST BE OF THE PRECAST TYPE AS SHOWN ON THE OPSD DRAWINGS 705.010 OR 705.020. DOUBLE CATCHBASIN ARE REQUIRED IN VALLEYS.
10. SINGLE CATCHBASIN LEADS SHALL BE PVC SDR35 PIPE WITH MINIMUM SLOPE OF 1% AND MINIMUM SIZES AS FOLLOWS:

-SINGLE CATCHBASIN	250mm DIAMETER
-DOUBLE CATCHBASIN	300mm DIAMETER
-REAR LOT CATCHBASIN	300mm DIAMETER
11. THE FRAME AND GRATE FOR CATCHBASINS SHALL BE OPSD 400.010, CATCHBASIN GRATES ARE TO BE RAMPED USING HOT-MIX ASPHALT. CATCHBASIN AT LOW POINTS SHALL BE SET TO BASE ASPHALT AND ADJUSTED TO SURFACE COURSE.
12. REINFORCED CONCRETE LEADS TO BE CONCRETE FRAME AND GRATE TO BE OPSD 400.010.
13. WHERE CATCHBASINS ARE CONNECTED DIRECTLY TO SEWERS, PRE-MANUFACTURED TEES SHALL BE USED.
14. RECTANGULAR BULK HEADS ARE REQUIRED AT MANHOLE CONNECTIONS TO EXISTING SEWERS.
15. MANHOLE STEPS ARE TO BE SOLID, RECTANGULAR ALUMINUM PER OPSD 405.020.
16. ROOF LEADERS SHALL DISCHARGE TO CONCRETE SPLASH PADS, UNLESS NOTED OTHERWISE.

1. WATERMAIN SHALL BE LOCATED AS SHOWN ON THE STANDARD TOWN ROADWAY CROSS SECTIONS. THIS LOCATION SHALL NORMALLY BE ON THE NORTH AND EAST SIDE OF THE STREET. WATERMAIN SHALL BE OFFSET FROM STREET LINE AS PER STANDARD ROAD CROSS SECTION.
2. ALL WATERMAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH OPSS 441. PIPE MATERIAL SHALL BE CLASS 150 PVC (DR18) IN ACCORDANCE WITH APPLICABLE AWWA STANDARDS, LATEST EDITION.
3. WATERMAIN BEDDING SHALL BE AS PER OPSS 802.010.
4. A MINIMUM CLEARANCE BETWEEN THE WATERMAIN AND ALL UTILITIES MUST BE KEPT, WHILE STILL MAINTAINING A MINIMUM DEPTH OF COVER AT ALL TIMES.
5. WATERMAIN SHALL BE INSTALLED WITH A MINIMUM COVER OF 1.8m.
6. CAST IRON MECHANICAL JOINT FITTINGS MEETING AWWA SPECIFICATIONS C-907 AND U.S.A. STANDARD 19.2 SHALL BE USED ON PVC PIPE.
7. ALL VALVES SHALL BE RESILIENT WEDGE GATE VALVES WITH VALVE BOX UNLESS OTHERWISE APPROVED BY THE TOWN. VALVES SHALL HAVE A NON-RISING STEM AND A 50mm SQUARE OPERATING NUT, OPENING COUNTER-CLOCKWISE.
8. VALVES SHALL BE 1.8m IN DEPTH SHALL SECURE A VALVE STEM EXTENSION.
9. A MINIMUM HORIZONTAL SEPARATION OF 2.5m SHALL BE MAINTAINED BETWEEN THE WATERMAIN AND ANY SEWER.
10. PIPE DEFLECTION SHOULD BE USED WHEREVER POSSIBLE TO MINIMIZE THE USE OF BENDS. WHEREVER IT IS NECESSARY TO DEFLECT FROM A STRAIGHT LINE, EITHER IN THE VERTICAL OR HORIZONTAL PLANE, THE AMOUNT OF DEFLECTION SHALL NOT EXCEED THE RECOMMENDATIONS OF THE MANUFACTURER.
11. THE TOWN ENGINEER MUST BE NOTIFIED AT LEAST 48 HOURS IN ADVANCE OF ANY PRESSURE TESTING. PRESSURE TESTING OF WATERMAINS MUST BE CARRIED OUT IN ACCORDANCE WITH TOWN SPECIFICATIONS AND OPSS 441.

1. EACH RESIDENTIAL UNIT SHALL HAVE A SEPARATE 25mm MINIMUM DIAMETER, TYPE K COPPER WATER SERVICE, WHEN SERVICE LINE EXCEEDS 30m IN LENGTH, THE SIZE OF THE LINE SHALL BE DETERMINED TO AN APPROPRIATE DIAMETER, OR AS DIRECTED BY THE TOWN.
2. SERVICES SHALL BE INSTALLED ACCORDING TO OPSD 110.010 AND 110.020.
3. WATER SERVICES TO BE LOCATED 2.5m FROM THE LOT LINE OPPOSITE THE DRIVEWAY WITH A CURB STOP AT THE STREET LINE AND A MINIMUM 1.0m CLEAR FROM THE EDGE OF DRIVEWAY. LOCATIONS TO BE MARKED WITH A WHITE PAINT.
4. THE MINIMUM DEPTH OF COVER IS 1.8m.
5. WATER SERVICES SHALL BE INSTALLED TO AVOID DRIVEWAYS, WHERE NOT POSSIBLE, FROST RINGS TO BE USED.
6. NO COUPLINGS WILL BE ALLOWED BETWEEN THE CURB STOP AND MAIN STOP.
7. STAINLESS STEEL SERVICE SADDLES SHALL BE USED WHEN TAPPING INTO THE PVC WATERMAIN.
8. A REMOTE READOUT WATER METER WILL BE REQUIRED ON EACH RESIDENCE, MAKE AND MODEL IS TO BE PER TOWN REQUIREMENTS COMPLETE WITH A REMOTE READER LOCATED ON AN OUTSIDE WALL ABOVE THE DRIVEWAY.

1. SANITARY SEWERS SHALL GENERALLY BE LOCATED 1.5m NORTH OR EAST OF THE ROAD CENTERLINE.
2. ALL POLYVINYL CHLORIDE (PVC) SANITARY SEWER PIPE SHALL BE CSA STANDARD B182.2, SCHEDULE 40. ALL SANITARY SEWERS SHALL HAVE WATERLOG JOINTS AND GASKETTED FITTINGS.
3. SEWERS SHALL BE CONSTRUCTED WITH BEDDING AS PER OPSD 802.010, AND 802.030, UNLESS OTHERWISE SPECIFIED BY A GEOTECHNICAL CONSULTANT AND APPROVED BY THE TOWN. IN WATER BEARING SAND, IF EXTENSIVE DEWATERING IS REQUIRED, A CLASS "A" MATERIAL CONSISTING OF CONCRETE OR GRAVEL SHALL BE USED.
4. FRAMES AND COVERS SHALL BE IN ACCORDANCE WITH O.P.S.D. 401.010, TYPE "A".
5. ALL DOMESTIC CONNECTIONS SHALL BE CONSTRUCTED IN ACCORDANCE WITH TOWN STANDARD DRAWINGS AND ALL APPLICABLE O.P.S.D. DETAILS TO A MAXIMUM DEPTH OF 4.5m. CONTROLLED SETTLEMENT JOINTS ARE REQUIRED ON ALL RISERS GREATER THAN 4.5m DEPTH.
6. INSTALL SEWER DROP CONNECTIONS TO MANHOLES AS PER OPSD 1003.010 AND 1003.020.
7. ALL MANHOLE ADJUSTMENT RINGS SHALL BE MORTARED FROM THE OUTSIDE AND INSIDE OF THE MANHOLE.
8. MANHOLES ARE TO BE WATER-PROOFED PER OPSD 1351. MANHOLES ARE TO BE WATERIGHT AND PARGED ALONG OUTSIDE OF JOINTS WITH MORTAR PRIOR TO BACKFILLING.
9. "CLAY PLUGS" SHALL BE USED IN THE TRENCH AND BE PLACED 2 TO 3 METERS UPSTREAM FROM ANY MANHOLE WHICH IS SUSPECTED OF BEING SUBJECT TO HIGH WATER LEVELS OF FLOODING. INFORMATION, SUPERVISOR VERIFICATION BY GEOTECHNICAL ENGINEER.
10. MAINTENANCE HOLE TOPS (FRAMES) ARE TO BE SET TO BASE COURSE ASPHALT GRADE, AND THEN ADJUSTED TO FINAL GRADE WHEN TOP LIFT OF ASPHALT IS PLACED. GRADE AND CROSSFALL ADJUSTMENT SHALL BE MADE USING PRODUCTS SPECIFICALLY MANUFACTURED FOR THIS PURPOSE. MODULAR ADJUSTMENT RINGS ARE TO BE USED TO ADJUST THE MANHOLE TO FINAL GRADE.
11. PRECAST CONCRETE MANHOLES SHALL CONFORM TO OPSD M701.010 (MODIFIED) FOR 1200mm DIAMETER.
 - a) REINFORCED, POURED IN PLACE CONCRETE MANHOLES, ARE SUBJECT TO APPROVAL OF THE TOWN ENGINEER.
 - b) SAFETY PLATFORMS ARE TO BE PROVIDED FOR MANHOLES IN ACCORDANCE WITH PROVINCIAL REGULATIONS.
12. MINIMUM CLEARANCES BETWEEN MANHOLES SHALL BE PROVIDED IN ACCORDANCE WITH M.O.E. GUIDELINES. MINIMUM CLEARANCE FOR SEWERS IN THE COMMON TRENCH SHALL BE 450mm BETWEEN THE OUTSIDE OF THE BELLS.
13. A MINIMUM COVER OF 2.8m BELOW THE FUTURE ROAD ELEVATION OR SUFFICIENT DEPTH FOR ALL TYPES OF FLOODING AND FROST COVER ARE TO BE PROVIDED. WHERE SEWERS ARE LOCATED WITHIN AN EASEMENT, A MINIMUM FROST COVER OF 1.5m MAY BE USED, PROVIDED SUCH SEWERS CROSS BELOW WATERMANS.
14. BULKHEADS OR PLUGS ARE REQUIRED IN DOWNSTREAM MANHOLE AT OUTLET FROM DEVELOPMENT, UNTIL FURTHER OCCUPANCY.
15. ALL TESTING OF THE SANITARY SYSTEM IS TO BE COMPLETED IN ACCORDANCE WITH THE "SANITARY SEWER INSPECTION AND TESTING AND ACCEPTANCE GUIDELINES"(OCT.2011) AS PREPARED BY THE REGION OF YORK, OR LATEST AMENDMENT THERETO.
16. MANHOLE STEPS ARE TO BE SOLID, RECTANGULAR ALUMINUM PER OPSD 405.020.

1. ALL SANITARY LATRINE PIPES SHALL BE PVC-DR28, GREEN IN COLOR. SINGLES ARE 100mm, DOUBLES ARE 125mm.
2. STORM SERVICES SHALL BE 150mm DIAMETER PVC DR 28, WHITE IN COLOR.
3. ELEVATION OF SERVICE CONNECTIONS AT THE STREETLINE IS TO BE 2.5m MINIMUM BELOW FINISHED GRADE AND TERMINATED WITH A WATER-PROOF PLUG.
4. LOCATION STAKED WITH A 2 x 4 MARKER PAINTED RED FOR SANITARY AND GREEN FOR STORM.
5. ALL LATRALS ARE TO BE INSPECTED BY CCTV FROM THE MAINLINE TO THE DWELLING AND A REPORT SUBMITTED TO THE TOWN PRIOR TO OCCUPANCY OF THE DWELLING.
6. TEST FITTINGS AT STREET-TO-LINE TO BE WATER-TIGHT PITCH FITTING.
7. BEDDING FOR LATRALS TO BE PER OPSD 1006.010 AND OPSD 1006.020 (MODIFIED).

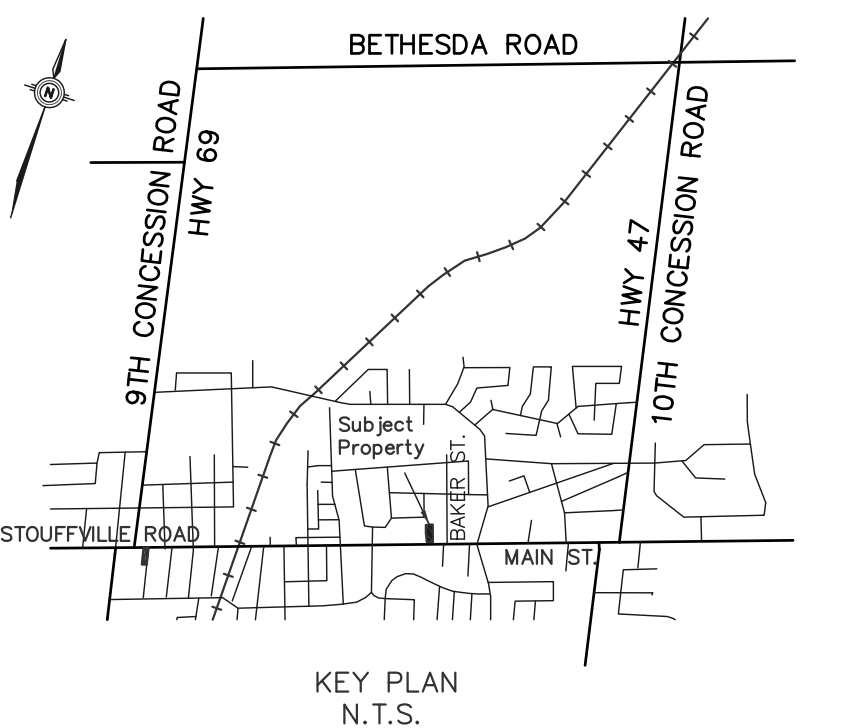
1. All external site areas disturbed by the activities of the contractor shall be restored to existing condition or better and to the satisfaction of the Town. Grassed areas shall be restored by placing 150mm topsoil and actively growing nursery sod. All boulevard to be seeded.
2. Topsoil in fill areas to be stripped. All fill materials approved for suitability by the geotechnical engineer prior to any filling or reuse of excavated material. Approval fill material shall be compacted to the satisfaction of the geotechnical engineer.
3. Pavement grades (min. 0.5%, max. 5.0%).
4. Drainage slopes with grades (min. 2%, max. 5.0%).
5. Slopes in landscaped areas and on berms shall not exceed 3 horizontal to 1 vertical with a maximum vertical elevation not in excess of the applicable Town standard.
6. Silt fence to be installed and maintained to prevent silt flowing onto adjacent lands prior to seeding topsoil and to be maintained until all construction is complete.
7. Construction access shall be constructed with a minimum depth of 450mm crushed stone base from the municipal curb or edge of pavement to the property line, to the satisfaction of the Town.
8. Boulevard grades are to be 2.0% to 4.0% max.
9. Situation control measures shall be in place prior to start of any construction.
10. All grading to conform to Town of Richmond Hill standards and specification and OPSD.
11. All construction to be carried out in accordance with the most recent design criteria, standards, and specifications of the Town of Richmond Hill and OPSD.
12. The geotechnical consultant shall verify the suitability of the engineered fill at source prior to hauling any material on site.
13. All fill shall be placed and compacted to 95% standard proctor density in maximum 0.20m lifts to sub-grade.

Diagram illustrating the RAIN WATER LEADER DETAIL. The cross-section shows the exterior face of the house, a typical rainwater leader, a detachable inverted 'Y' fitting for over-flow, a clean-out, a concrete splashpad, and the finished grade. The trench leading to the infiltration trench is specified as 600mm MIN. The final detail is labeled RAIN WATER LEADER DETAIL N.T.S.

$$\begin{aligned} \text{REQ'D. VOLUME (V)} &= \frac{\text{REAR ROOF AREAS (sq.m.)} \times 0.025\text{m}}{0.4 \text{ (Void Ratio)}} \\ V &= \frac{350 \text{ (sq.m.)} \times 0.025}{0.4} \\ V &= 21.8 \text{ cu.m.} \end{aligned}$$

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ROAD ALLOWANCE



NOTE:
Distances Shown On This Plan Are In Metres And Can Be Converted To Feet By Dividing By 0.3048
The Position of Pole Lines, Conduits, Watermains, Sewers and Other Underground and Overground Utilities and Structures is Not Necessarily Shown On The Contract Drawing. And Where Shown, The Accuracy Of The Position of Such Utilities and Structures is Not Guaranteed Before Starting Work, The Contractor Shall Inform Himself Of The Exact Location of All Such Utilities and Structures, and Shall Assume All Liability For Damage To Them.

266.35.1

	PROPOSED ELEVATION
	EX. ELEVATION
	EX. STORM MAINTENANCE HOLE
	EX. SANITARY MAINTENANCE HOLE
	PROP. ROOF LEADER C/W SPLASH PAD
☐ Ex.CB	EXISTING CATCHBASIN
☐ CB	PROPOSED CATCHBASIN
	PROP. WATER METER CHAMBER
	PROPOSED WATER BOX
	PROP. HYDRANT & VALVE
	PROP. FLOW DIRECTION
	PROP. 3:1 (MAX) SLOPE
FF	FINISHED FIRST FLOOR ELEVATION
TW	TOP OF FOUNDATION WALL ELEVATION
USF	UNDERSIDE OF FOUNDATION ELEVATION
	DENOTES EXISTING TREES
	STOP SIGN C/W STOP BAR
	OVERLAND FLOW ROUTE

[illegible]

2426009 ONTARIO LTD.
PROPOSED RESIDENTIAL DEVELOPMENT
6460 MAIN STREET, TOWN OF WHITCHURCH-STOUFFVILLE

CONSULTANT 	ACCEPTED TOWN OF WHITCHURCH STOUFFVILLE _____ SIGNATURE DATE
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 **TOWN OF
WHITCHURCH-STOUFFVILLE**

 **AME-TECH Developments Limited**
Urban Engineering & Project Management Services
346 Newkirk Road, Unit 9, Richmond Hill, Ontario L4C 0A9, 905.884.4336

LOT 30 AND PART OF LOT 31, REGISTERED PLAN 230
TOWN OF WHITCHURCH-STOUFFVILLE
REGIONAL MUNICIPALITY OF YORK

Surveyed by: BIAWCOLLI, O.L.S.		Checked by: GDR	Town File No.
Drawn by: CADD		Approved by: CDR	SPA20.019
Designed by: CDR		Date: MARCH 2024	Drawing No.
Scale		SG1	
0m 5m 10m Horz. 1:250 		Sheet No.	
		1 OF 2	